

WILSON DEVELOPMENTS INC.

FUNCTIONAL SERVICING REPORT

EAST RIDGE STORAGE SITE
MUNICIPALITY OF BROCKTON

JUNE 2022

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TABLE OF CONTENTS

1.	INTRODUCTION	1
1.1	Location	1
1.2	Development Proposal	1
2.	WATER SERVICING.....	3
3.	SANITARY SEWER SERVICING.....	4
4.	STORM SEWER SYSTEM.....	5
4.1	Design Requirements	5
4.2	SWM Facility Characteristics	5
4.2.1	SWM Facility Performance.....	6
4.3	Modelling Results	6
5.	GRADING & EROSION AND SEDIMENT CONTROL	8
5.1	Construction Stage	8
6.	UTILITIES.....	9
6.1	Electricity	9
6.2	Natural Gas	9
6.3	Telephone/ Cable TV/ Internet.....	9

LIST OF TABLES

Table 6.1 – SWM Facility Geometry.....	5
Table 6.2 – SWM Facility Performance.....	6
Table 6.3 - Peak Flow Summary	6

LIST OF FIGURES

Figure 1 - Site Location	1
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APPENDICES

A – Drawings	
SP1 – Proposed Site Plan	
SS1 – Site Servicing Plan	
B – Stormwater Modelling	

1. INTRODUCTION

Cobide Engineering Inc. was retained by Wilson Developments Inc. to provide engineering services in support of a Site Plan Approval Application. The application will be to develop the site into a storage facility with 50 external self storage units and two (2) buildings that can have a varying amount of units as the internal walls are modular and can be modified based on the requirements of the tenant.

A copy of the proposed Site Plan has been included in Appendix A as Drawing SP1.

1.1 LOCATION

The proposed development is located Part 1 of Plan 3R-10368, Former Town of Walkerton, Municipality of Brockton, County of Bruce (described herein as the “site”). A Site Location Map is included below as Figure 1. The subject property is approximately 0.40 hectares in area.



Figure 1 - Site Location

1.2 DEVELOPMENT PROPOSAL

The development will consist of external exterior storage units that are elevated and sit above the ground as well as two permanent structures as well as the associated parking and driving lanes. Below are pictures of the exterior units. As you can see in the pictures, the units are elevated to allow runoff to pass beneath the structure.

The site will be accessed from Ontario Road.

Functional Servicing Report
East Ridge Storage Site



2. WATER SERVICING

The proposed development will be serviced from the existing 25mm dia. water service that is currently stubbed at property line with a curb stop valve. It is proposed that the 25mm dia. service is extended into the most easterly building first, then a 25mm dia. water service will be connected between the two (2) buildings (as shown in Appendix A as Drawing SS1).

3. SANITARY SEWER SERVICING

The proposed development will incorporate connecting to the existing 150mm dia. sanitary service that is currently stubbed at property line with a cleanout. The proposed development will incorporate a 150mm dia. sanitary sewer main that is located within the travelled lane of the development.

Each of the two (2) main buildings will be serviced with a 125mm dia. service lateral (as shown in Appendix A as Drawing SS1).

4. STORM SEWER SYSTEM

The subject property is currently vacant. The site is generally sloping from south to north, and west to east. There are no existing storm sewers on the property., The site discharges into an existing ditch on the west side of Ontario Road. Ontario Road will be considered Discharge Point #1 for the purposes of this report.

The proposed development will be graded such that runoff is conveyed via swales and sheet flow to a new dry stormwater management pond along the north side of the property. The outlet for the stormwater management pond will consist of a headwall, and a 250mm dia. storm sewer c/w an orifice, that will then discharge into the existing ditch on the west side of Ontario Road.

The hydrologic modelling software PCSWMM Version 5.6.1803 Professional 2D was used to determine the pre and post-development peak flows of the 2 yr., 5 yr., 25 yr., 50 yr., and 100 yr. storm events (6 hour duration, SCS Type II, AMC II storm, Mount Forest IDF Parameters).

The pre-development and post development parameters and model outputs are contained in Appendix B.

4.1 DESIGN REQUIREMENTS

The intent of stormwater quantity control is to limit the flows under proposed conditions to existing levels or less to protect the downstream watercourses, infrastructure and properties.

Minor and Major flows from the majority of the development will be conveyed to the proposed stormwater management facility via asphalt swale overland flow routes.

Due to the increase in impervious area, stormwater quantity control will be required for the site. The design of the stormwater management facility has assumed a free outlet from the storage facility.

4.2 SWM FACILITY CHARACTERISTICS

The stormwater management facility and outlet structure have been designed to control peak runoff rates as well as conform to MECP best practices.

A 50 mm orifice will be installed on the headwall to control peak runoff rates.

In order to provide the above required volumes and discharges, the following SWM Facility geometry is being proposed:

Table 6.1 – SWM Facility Geometry

SWM FACILITY	DETAILED DESIGN
Side Slope	3:1
SWM Facility Bottom	297.78
Top Elevation	298.84 m
High Water Elevation	298.65 m

The outlet configuration for the SWM Facility will be as follows:

- A 250mm diameter storm sewer with a 50mm orifice and an outlet elevation of 297.78 m;

As seen by the proposed inverts, the proposed stormwater management facility will be constructed as a drypond.

4.2.1 SWM FACILITY PERFORMANCE

Below is a summary of the hydraulic performance of the stormwater SWM Facility during the various storm events.

Table 6.2 – SWM Facility Performance

RETURN PERIOD	ELEVATION (m)	STORAGE (m ³)	DISCHARGE (l/s)
2 Year	298.37	56	4.3
5 Year	298.47	83	4.6
25 Year	298.58	129	5.0
50 Year	298.62	151	5.1
100 Year	298.65	172	5.2

4.3 MODELLING RESULTS

Based upon the above outlet structure, the following summarizes the pre-development and post development peak flows to the discharge point.

Table 6.3 - Peak Flow Summary

RETURN PERIOD	DISCHARGE POINT #1 (L/S)	
	PRE	POST
2 Year	3.0	4.3
5 Year	5.3	4.6
25 Year	10.3	5.0
50 Year	12.9	5.1
100 Year	15.8	5.2

As seen in the above table, the post development peak flows will be less the pre development peak flows for all design storm events at Discharge Point #1 except the 2 year design storm event. The exceedance is very small and not expected to cause any issues downstream. The infrastructure within the development would have been designed to convey the runoff from either the 5 or 25 year storm event which the post development flows are lower than.

5. GRADING & EROSION AND SEDIMENT CONTROL

Erosion and sediment controls shall meet the requirements of the most recent version of the MOE *Stormwater Management Planning and Design Manual* at the time of construction.

5.1 CONSTRUCTION STAGE

Prior to the start of construction, appropriate sediment control facilities are to be in place. Following are details regarding erosion and sediment control that are to be implemented:

- Placement of Light Duty Siltation fence will also be installed at any development grading limits where runoff may discharge from the site;
- Mud mat will be placed at construction access to keep public roadways free from debris during the construction period;
- Re-vegetate all disturbed areas after underground and surface works have been constructed.

Prior to removal of sediment control facilities, ensure that sediment that may have accumulated has been removed.

Once the area has been stabilized, the silt fencing can be removed.

6. UTILITIES

6.1 ELECTRICITY

Westario Power Inc. will be responsible for completing the design of the electrical distribution system. Each of the two (2) buildings will be individually serviced from an on site padmount transformer. There is no electricity to the exterior units. Underground distribution lines will be utilized for this development.

6.2 NATURAL GAS

Union Gas will be responsible for completing the design of the natural gas distribution system. Each of the two (2) buildings will be serviced.

6.3 TELEPHONE/ CABLE TV/ INTERNET

Wightman and Eastlink will be given the opportunity to provide telephone, cable TV and internet services to the development. They will complete their own design.

Sincerely,

Cobide Engineering Inc.



Travis Burnside, P. Eng.

H:\Wilson\03705 - East Ridge Storage Site\Reports\FSR\2022-05-31 FSR\2022-05-31 East Ridge Storage Site FSR 03705.docx

Appendix A

DRAWINGS

FUNCTIONAL SERVICING REPORT

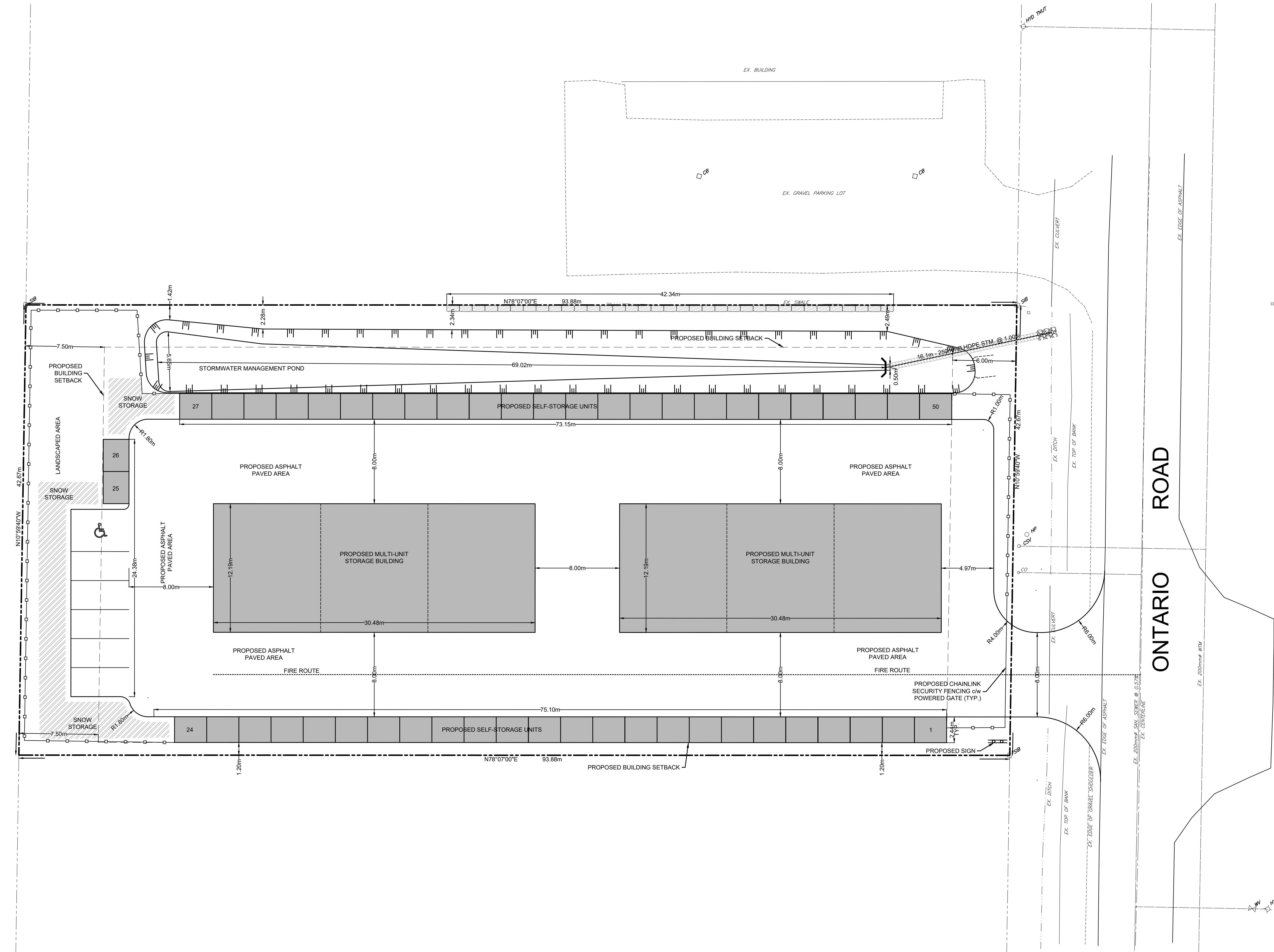
EAST RIDGE STORAGE SITE

MUNICIPALITY OF BROCKTON

CAUTION:
THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THIS DRAWING. AND, WHILE THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED, BEFORE STARTING WORK, THE CONTRACTOR SHALL INFORM HIMSELF OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

Notes

- PROPERTY BOUNDARY DERIVED FROM INFORMATION SHOWN ON PLAN OF SURVEY BY HEWETT AND MILNE LIMITED, DATED JULY 15, 2020.
- PROPOSED PROFILE GRADE DERIVED FROM SURVEY BY COBIDE ENGINEERING INC., COMPLETED ON NOVEMBER 25, 2020.
- ALL ORGANIC MATERIAL WITHIN 1.2m OF FINISHED PROFILE GRADE TO BE REMOVED FROM ALL AREAS UNDER THE TRAVELED PORTION OF THE ROAD.
- FIELD LOCATES OF ALL UNDERGROUND UTILITIES INCLUDING BUT NOT LIMITED TO WATER, GAS, HYDRO, TELEPHONE, AND CABLE TELEVISION SHALL BE ARRANGED PRIOR TO CONSTRUCTION AND IS THEREFORE RESPONSIBILITY OF THE CONTRACTOR.
- THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION PURPOSES UNTIL STAMPED ISSUED FOR CONSTRUCTION.
- ALL CONSTRUCTION SHALL BE COMPLETED IN ACCORDANCE WITH THE MUNICIPALITY OF BROCKTON MUNICIPAL DEVELOPMENT AND SERVICING GUIDELINES.



Benchmark Information

BM1: TOP NUT ON FIRE HYDRANT ON EAST SIDE OF ONTARIO ROAD, APPROXIMATELY 14.5m SOUTH OF THE SUBJECT PROPERTY, ELEVATION 299.32m

BM2: SIB LOCATED ON THE MOST NORTHEASTERN CORNER OF THE SUBJECT PROPERTY (ALONG ONTARIO ROAD), ELEVATION 297.80m

1	JUNE 9/22	SECOND SUBMISSION	KW	TLB
0	FEB 24/22	PRELIMINARY SUBMISSION	KW	TLB
No.	DATE	DESCRIPTION	BY	APPD

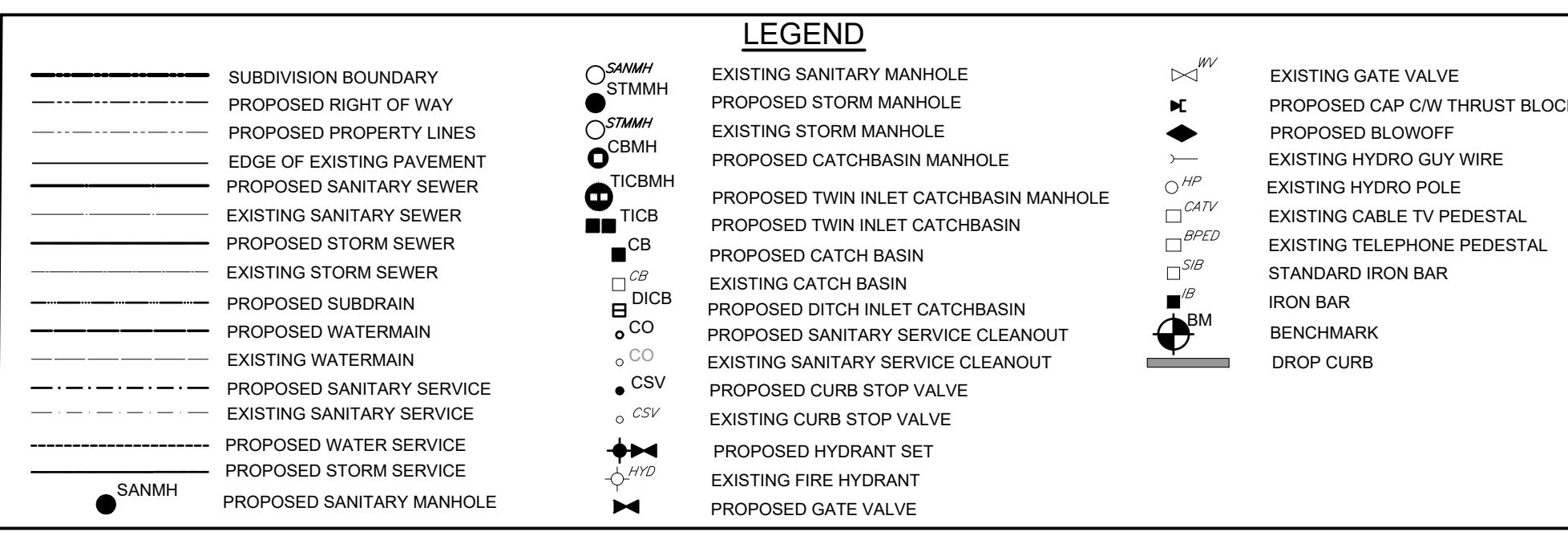
REVISION / ISSUE

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COBIDE
ENGINEERING INC.
517 - 10th STREET, Hanover, Ontario N4N 1R4
Telephone: (519) 506-5959
www.cobideeng.com

Title: EAST RIDGE STORAGE SITE PROPOSED DEVELOPMENT TOWN OF WALKERTON MUNICIPALITY OF BROCKTON DEVELOPMENT SITE PLAN
Client: BRAD WILSON
Design: TLB/KW **Scale:** 1:200
Drawn: KW **Approved:**
Checked: TLB
Date: JUNE 2022 **Design Engineer:**
DRAWING No. 03705-SP1



PROPOSED SELF-STORAGE DEVELOPMENT STATISTICS			
REGULATION	REQUIRED	PROVIDED	RELIEF REQUIRED
MIN. LOT AREA	750m ²	4005.38m ²	NO
MIN. FRONT YARD	3.0m	42.67m	NO
MIN. EXTERIOR SIDE YARD	6.0m	6.0m	NO
MIN. INTERIOR SIDE YARD	1.2m/4.0m	1.2m/8.39m	NO
MIN. REAR YARD	7.5m	7.5m	NO
MAX. LOT COVERAGE	75%	27.84%	NO
MAX. BUILDING HEIGHT	11.0m	<11.0m	NO
PARKING	6 SPACES	6 SPACES	NO

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No.	DATE	DESCRIPTION	BY	APPD

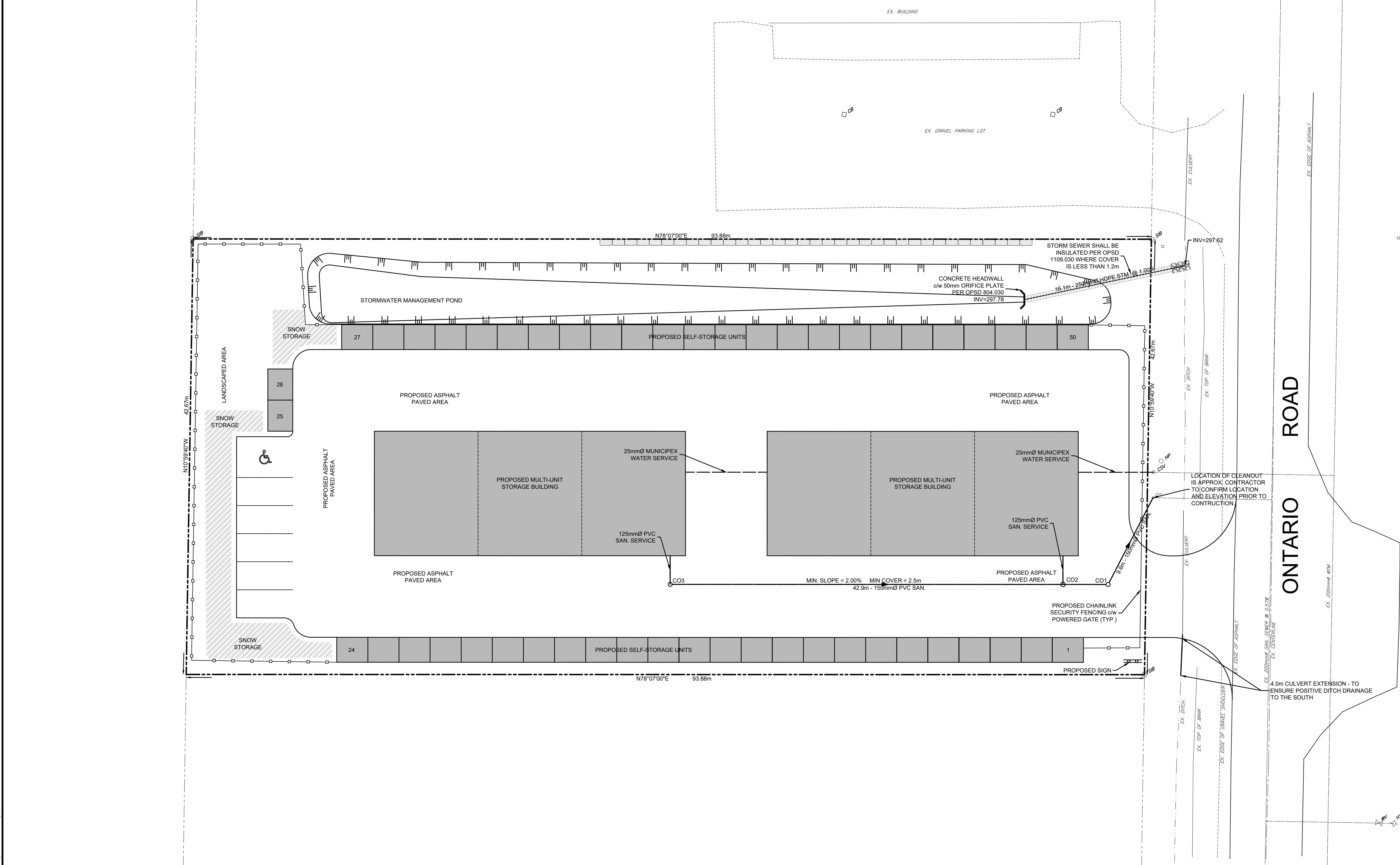
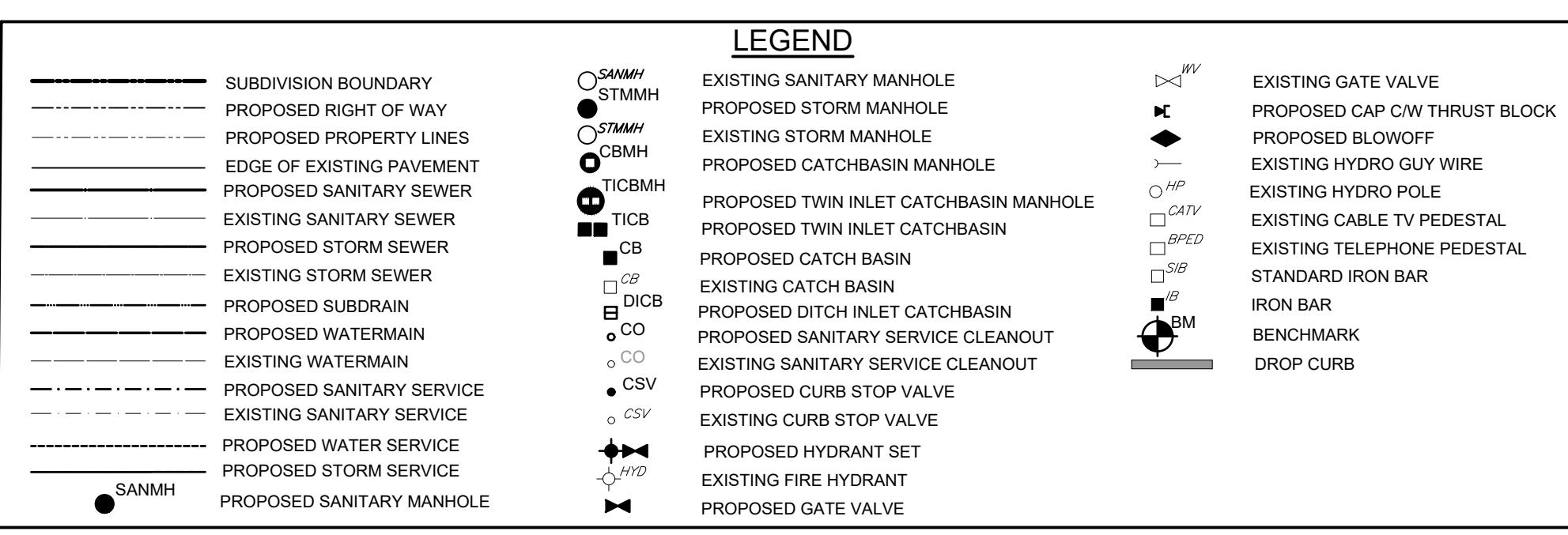
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Client:	BRAD WILSON		
Design:	KW	Scale:	1:200
Drawn:	KW	Approved:	
Checked:	TLB		
Date:	JUNE 2022		
Design Engineer			
DRAWING No.	03705-SS1		



Appendix B

MODEL PARAMETERS AND OUTPUT

STORMWATER MANAGEMENT REPORT

EAST RIDGE STORAGE SITE

MUNICIPALITY OF BROCKTON

Table B.1 Parameter Summary Table

Proposed Conditions										
Outlet Location	Model Catchment ID	Description	Area (ha)	Drainage Channel (m)	Flow Length (m)	Gradient (%)	Total Imperv. (%)	Not Connected Imperv. (%)	Manning's 'n' (Perv.)	CN (Perv.)
	101	Site - Pre Development	0.40	85	47	1.0	0.0	#DIV/0!	0.30	72.0
	201	Site - Post Development	0.40	85	47	1.0	68.4	0.0	0.25	77.0

Table B.2 Site Soils: (as per Ontario Soil Survey Report for Bruce County)

Soil Type
Harriston Loam

Hydrologic Soil Group
BC

TABLE OF CURVE NUMBERS (CN's)								
Land Use	Hydrologic Soil Type							
	A	AB	B	BC	C	CD	D	Manning's 'n'
Meadow	50	54	58	64.5	71	74.5	78	0.4
Woodlot	50	55.3	60.5	67	73.5	76.8	80	0.4
Long Grass	55	60	65	72	79	81.5	84	0.3
Lawns	60	65.5	71	77	83	86	89	0.25
Pasture/Range	58	61.5	65	70.5	76	78.5	81	0.17
Crop	66	70	74	78	82	84	86	0.13
Fallow (bare)	77	82	86	89	91	93	94	0.05
Built-up	60	65.5	71	77	83	89	89	0.25
Streets, paved	98	98	98	98	98	98	98	0.01

continuous grass
forests
natural, not maintained
maintained
farm pasture
farm land
idle farm land (bare)
Lawns Proposed

HYDROLOGIC SOIL TYPE (%) - Proposed Conditions								
Catchment	Hydrologic Soil Type							
	A	AB	B	BC	C	CD	D	TOTAL
201	0	0	0	100	0	0	0	100
202	0	0	0	100	0	0	0	100

LAND USE (%) - Proposed Conditions										
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Imperv. Not Connected (Rooftops)	Imperv. Connected	Total
201	0	0	100	0.0	0	0	0	0.0	0.0	100
202	0	0	0	32	0	0	0	0.0	68.4	100

CURVE NUMBER (CN) - Proposed Conditions											
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Built-up	Imperv. Not Connected (Rooftops)	Weighted CN - Pervious	Manning's 'n'
201	65	67	72	77	70.5	78	89	77	90	72.0	0.30
202	65	67.0	72	77	70.5	78	89	77.0	90	77.0	0.25

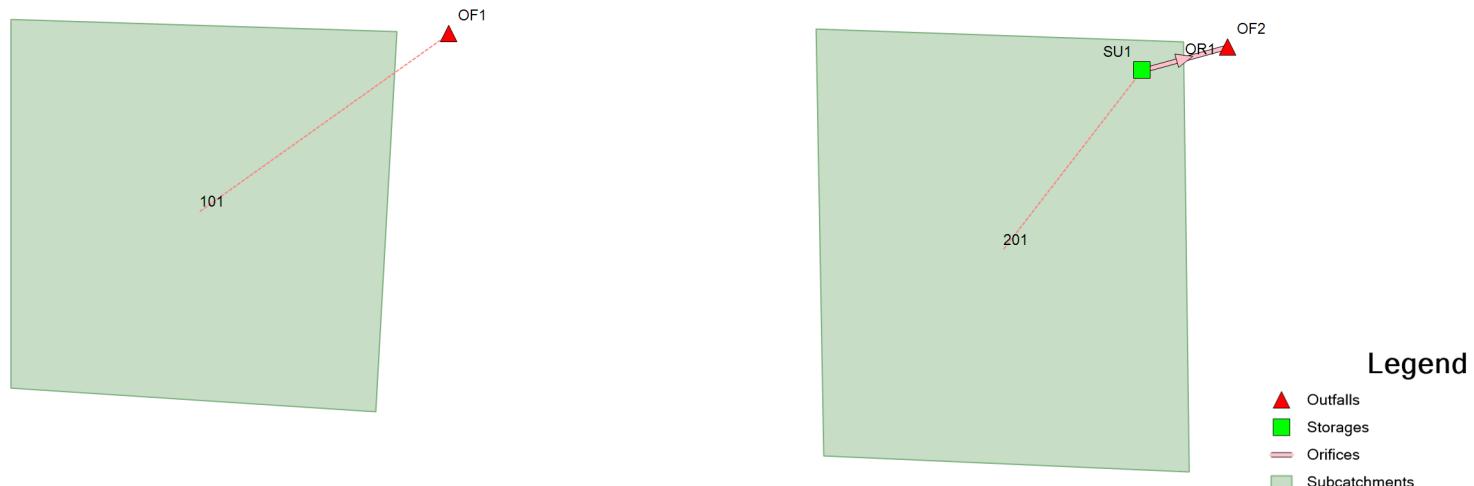
Table A.3: Impervious Area Determination for Subcatchments 201 - 204

Proposed Conditions						
Area of Concern	Total Area (ha)	Impervious Area Connected		Impervious Area Not Connected (Rooftops)		Total (%)
		(ha)	(%)	(ha)	(%)	
201	0.40	0.00	0.0	0.00	0.0	0.0
202	0.40	0.27	68.4	0.00	0.0	68.4

Table B.3 - Impervious Area Determination for Proposed Catchments 201 - 204

Catchment				Imperv. Area	Imperv %
201	m of	0	m wide ROW @ 57% imperv.	0.00 ha	0.0 %
	1 Impervious Area	0	m ² @ 100% imperv.	0.00 ha	0.0 %
	Roof Area	0	m ² @ 100% imperv.	0.00 ha	0.0 %
	Roof Area	0	m ² @ 100% imperv.	0.00 ha	0.0 %
				0.00 ha	
202	m of	0	m wide ROW @ 57% imperv.	0.00 ha	0.0 %
	Impervious Area	0	m ² @ 100% imperv.	0.00 ha	0.0 %
	1 Roof Area	2741	m ² @ 100% imperv.	0.27 ha	68.4 %
	Roof Area	0	m ² @ 100% imperv.	0.00 ha	0.0 %
				0.00 ha	

WILSON DEVELOPMENTS - EAST RIDGE STORAGE SITE - MODEL SCHEMATIC



WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – MODEL DETAILS

[TITLE]
[OPTIONS]

<code>;;Options</code>	<code>Value</code>
<code>;;</code>	
FLOW_UNITS	LPS
INFILTRATION	CURVE_NUMBER
FLOW_ROUTING	DYNWAVE
START_DATE	12/24/2021
START_TIME	00:00
REPORT_START_DATE	12/24/2021
REPORT_START_TIME	00:00
END_DATE	12/25/2021
END_TIME	00:00
SWEET_START	1/1
SWEET_END	12/31
DRY_DAYS	0
REPORT_STEP	00:01:00
WET_STEP	00:05:00
DRY_STEP	00:05:00
ROUTING_STEP	5
ALLOW_PONDING	NO
INERTIAL_DAMPING	PARTIAL
VARIABLE_STEP	0.75
LENGTHENING_STEP	0
MIN_SURFAREA	0
NORMAL_FLOW_LIMITED	BOTH
SKIP_STEADY_STATE	NO
FORCE_MAIN_EQUATION	H-W
LINK_OFFSETS	ELEVATION
MIN_SLOPE	0
MAX_TRIALS	8
HEAD_TOLERANCE	0
SYS_FLOW_TOL	5
LAT_FLOW_TOL	5
MINIMUM_STEP	0.5
THREADS	2

[EVAPORATION]

<code>;;Type</code>	<code>Parameters</code>
<code>;;</code>	
CONSTANT	0.0
DRY_ONLY	NO

[RAINGAGES]

<code>;;</code>	<code>Rain</code>	<code>Time</code>	<code>Snow</code>	<code>Data</code>
<code>;;Name</code>	<code>Type</code>	<code>Intrvl</code>	<code>Catch</code>	<code>Source</code>
<code>;;</code>				
SCS_6h_38.8mm_2yr	INTENSITY	0:05	1.0	TIMESERIES SCS_6h_38.8mm_2yr
SCS_6h_49.4mm_5yr	INTENSITY	0:05	1.0	TIMESERIES SCS_6h_49.4mm_5yr
SCS_6h_65.3mm_25yr	INTENSITY	0:05	1.0	TIMESERIES SCS_6h_65.3mm_25yr
SCS_6h_71.9mm_50yr	INTENSITY	0:05	1.0	TIMESERIES SCS_6h_71.9mm_50yr
SCS_6h_78.4mm_100yr	INTENSITY	0:05	1.0	TIMESERIES SCS_6h_78.4mm_100yr

[SUBCATCHMENTS]

<code>;;</code>	<code>Raingage</code>	<code>Outlet</code>	<code>Total Area</code>	<code>Pcnt. Imperv</code>	<code>Width</code>	<code>Pcnt. Slope</code>	<code>Curb Length</code>	<code>Snow Pack</code>
<code>;;Name</code>								
<code>;;</code>								
101	SCS_6h_78.4mm_100yr	OF1	0.4	0	85	1	0	
201	SCS_6h_78.4mm_100yr	SU1	0.4	68.4	85	1	0	

[SUBAREAS]

<code>;;Subcatchment</code>	<code>N-Imperc</code>	<code>N-Perv</code>	<code>S-Imperc</code>	<code>S-Perv</code>	<code>PctZero</code>	<code>RouteTo</code>	<code>PctRouted</code>
<code>;;</code>							
<code>;;</code>							
101	0.01	0.3	0.05	0.05	25	OUTLET	
201	0.01	0.25	0.05	0.05	25	OUTLET	

[INFILTRATION]

<code>;;Subcatchment</code>	<code>CurveNum</code>	<code>HydCon</code>	<code>DryTime</code>
<code>;;</code>			
101	72	0.5	7
201	77	0.5	7

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – MODEL DETAILS

[OUTFALLS]

<i>;</i>	<i>Invert</i>	<i>Outfall</i>	<i>Stage/Table</i>	<i>Tide</i>
<i>;</i> <i>Name</i>	<i>Elev.</i>	<i>Type</i>	<i>Time Series</i>	<i>Gate</i> <i>Route</i> <i>To</i>
OF1	0	FREE		NO
OF2	0	FREE		NO

[STORAGE]

<i>;</i>	<i>Invert</i>	<i>Max.</i>	<i>Init.</i>	<i>Storage</i>	<i>Curve</i>	<i>Ponded</i>	<i>Evap.</i>
<i>;</i> <i>Name</i>	<i>Elev.</i>	<i>Depth</i>	<i>Depth</i>	<i>Curve</i>	<i>Params</i>	<i>Area</i>	<i>Frac.</i>
<i>Infiltration parameters</i>							
SU1	297.78	1.06	0	TABULAR	Pond	0	0

[ORIFICES]

<i>;</i>	<i>Inlet</i>	<i>Outlet</i>	<i>Orifice</i>	<i>Crest</i>	<i>Disch.</i>	<i>Flap</i>	<i>Open/Close</i>
<i>;</i> <i>Name</i>	<i>Node</i>	<i>Node</i>	<i>Type</i>	<i>Height</i>	<i>Coeff.</i>	<i>Gate</i>	<i>Time</i>
OR1	SU1	OF2	SIDE	297.78	0.65	NO	0

[XSECTIONS]

<i>;</i> <i>Link</i>	<i>Shape</i>	<i>Geom1</i>	<i>Geom2</i>	<i>Geom3</i>	<i>Geom4</i>	<i>Barrels</i>
OR1	CIRCULAR	0.05	0	0	0	

[CURVES]

<i>;</i> <i>Name</i>	<i>Type</i>	<i>X-Value</i>	<i>Y-Value</i>
Pond	Storage	0	0
Pond		.22	40
Pond		.47	165
Pond		.72	355
Pond		.97	900

[TIMESERIES]

<i>;</i> <i>Name</i>	<i>Date</i>	<i>Time</i>	<i>Value</i>
<i>;SCS_6h_38.8mm design storm, total rainfall = 38.8 mm, rain units = mm/hr.</i>			
<i>SCS_6h_38.8mm_2yr</i>			
<i>;SCS_6h_49.4mm design storm, total rainfall = 49.4 mm, rain units = mm/hr.</i>			
<i>SCS_6h_49.4mm_5yr</i>			
<i>;SCS_6h_65.3mm design storm, total rainfall = 65.3 mm, rain units = mm/hr.</i>			
<i>SCS_6h_65.3mm_25yr</i>			
<i>;SCS_6h_71.9mm design storm, total rainfall = 71.9 mm, rain units = mm/hr.</i>			
<i>SCS_6h_71.9mm_50yr</i>			
<i>;SCS_6h_78.4mm design storm, total rainfall = 78.4 mm, rain units = mm/hr.</i>			
<i>SCS_6h_78.4mm_100yr</i>			

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 2 YR DESIGN STORM EVENT

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

Element Count

Number of rain gages 5
Number of subcatchments ... 2
Number of nodes 3
Number of links 1
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_6h_38.8mm_2yr	SCS_6h_38.8mm_2yr	INTENSITY	5 min.
SCS_6h_49.4mm_5yr	SCS_6h_49.4mm_5yr	INTENSITY	5 min.
SCS_6h_65.3mm_25yr	SCS_6h_65.3mm_25yr	INTENSITY	5 min.
SCS_6h_71.9mm_50yr	SCS_6h_71.9mm_50yr	INTENSITY	5 min.
SCS_6h_78.4mm_100yr	SCS_6h_78.4mm_100yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	0.40	85.00	0.00	1.0000	SCS_6h_38.8mm_2yr	OF1
201	0.40	85.00	68.40	1.0000	SCS_6h_38.8mm_2yr	SU1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	297.78	1.06	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
OR1	SU1	OF2	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
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NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 2 YR DESIGN STORM EVENT

 Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 12/24/2021 00:00:00
 Ending Date 12/25/2021 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.031	38.807
Evaporation Loss	0.000	0.000
Infiltration Loss	0.015	18.401
Surface Runoff	0.016	20.310
Final Storage	0.000	0.117
Continuity Error (%)	-0.054	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.016	0.162
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.016	0.162
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 2 YR DESIGN STORM EVENT

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
101	38.81	0.00	0.00	28.69	9.94	0.04	2.96	0.256
201	38.81	0.00	0.00	8.11	30.68	0.12	24.72	0.791

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.17	0.59	298.37	0 04:00	0.59

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow 10^6 ltr	Total Inflow 10^6 ltr	Flow Balance Volume	Error Percent
OF1	OUTFALL	2.96	2.96	0 02:45	0.0397	0.0397	0.000	0.000
OF2	OUTFALL	0.00	4.26	0 04:00	0	0.123	0.000	0.000
SU1	STORAGE	24.72	24.72	0 02:25	0.123	0.123	0.000	0.000

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt	Exfil Pcnt	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.012	4	0	0	0.056	16	0 04:00	4.26

Outfall Loading Summary

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 2 YR DESIGN STORM EVENT

Outfall Node	Flow	Avg	Max	Total
	Freq Pcnt	Flow LPS	Flow LPS	Volume 10^6 ltr
OF1	62.49	0.73	2.96	0.040
OF2	43.04	3.30	4.26	0.123
System	52.76	4.02	7.14	0.162

Link Flow Summary

Link	Type	Maximum	Time of Max	Maximum	Max/	Max/
		Flow LPS	Occurrence days hr:min	Veloc m/sec	Full Flow	Full Depth
OR1	ORIFICE	4.26	0 04:00			1.00

Flow Classification Summary

Conduit	Adjusted Length	Fraction of Time in Flow Class								
		/Actual Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Crit	Inlet Ltd
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Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Mar 04 16:24:18 2022

Analysis ended on: Fri Mar 04 16:24:19 2022

Total elapsed time: 00:00:01

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 5 YR DESIGN STORM EVENT

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

Element Count

Number of rain gages 5
Number of subcatchments ... 2
Number of nodes 3
Number of links 1
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_6h_38.8mm_2yr	SCS_6h_38.8mm_2yr	INTENSITY	5 min.
SCS_6h_49.4mm_5yr	SCS_6h_49.4mm_5yr	INTENSITY	5 min.
SCS_6h_65.3mm_25yr	SCS_6h_65.3mm_25yr	INTENSITY	5 min.
SCS_6h_71.9mm_50yr	SCS_6h_71.9mm_50yr	INTENSITY	5 min.
SCS_6h_78.4mm_100yr	SCS_6h_78.4mm_100yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	0.40	85.00	0.00	1.0000	SCS_6h_49.4mm_5yr	OF1
201	0.40	85.00	68.40	1.0000	SCS_6h_49.4mm_5yr	SU1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	297.78	1.06	0.0	

Name	From Node	To Node	Type	Length	%Slope	Roughness
OR1	SU1	OF2	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 5 YR DESIGN STORM EVENT

 Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 12/24/2021 00:00:00
 Ending Date 12/25/2021 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.040	49.408
Evaporation Loss	0.000	0.000
Infiltration Loss	0.017	21.784
Surface Runoff	0.022	27.534
Final Storage	0.000	0.117
Continuity Error (%)	-0.055	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.022	0.220
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.022	0.220
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 5 YR DESIGN STORM EVENT

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
101	49.41	0.00	0.00	34.09	15.14	0.06	5.33	0.306
201	49.41	0.00	0.00	9.48	39.93	0.16	32.48	0.808

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.24	0.69	298.47	0 04:32	0.69

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow 10^6 ltr	Total Inflow 10^6 ltr	Flow Balance Volume	Flow Error Percent
OF1	OUTFALL	5.33	5.33	0 02:45	0.0605	0.0605	0.000	0.000
OF2	OUTFALL	0.00	4.60	0 04:32	0	0.16	0.000	0.000
SU1	STORAGE	32.48	32.48	0 02:25	0.16	0.16	0.16	0.000

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt	Exfil Pcnt	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.021	6	0	0	0.083	24	0 04:32	4.60

Outfall Loading Summary

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 5 YR DESIGN STORM EVENT

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	63.49	1.09	5.33	0.061
OF2	51.06	3.62	4.60	0.160
System	57.28	4.71	9.75	0.220

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
OR1	ORIFICE	4.60	0 04:32		1.00	

Flow Classification Summary

Conduit	Adjusted Length	Fraction of Time in Flow Class							
		/Actual Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Crit

Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Mar 04 16:24:54 2022

Analysis ended on: Fri Mar 04 16:24:54 2022

Total elapsed time: < 1 sec

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 25 YR DESIGN STORM EVENT

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

Element Count

Number of rain gages	5
Number of subcatchments ...	2
Number of nodes	3
Number of links	1
Number of pollutants	0
Number of land uses	0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_6h_38.8mm_2yr	SCS_6h_38.8mm_2yr	INTENSITY	5 min.
SCS_6h_49.4mm_5yr	SCS_6h_49.4mm_5yr	INTENSITY	5 min.
SCS_6h_65.3mm_25yr	SCS_6h_65.3mm_25yr	INTENSITY	5 min.
SCS_6h_71.9mm_50yr	SCS_6h_71.9mm_50yr	INTENSITY	5 min.
SCS_6h_78.4mm_100yr	SCS_6h_78.4mm_100yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	0.40	85.00	0.00	1.0000	SCS_6h_65.3mm_25yr	OF1
201	0.40	85.00	68.40	1.0000	SCS_6h_65.3mm_25yr	SU1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	297.78	1.06	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
OR1	SU1	OF2	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
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NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 25 YR DESIGN STORM EVENT

 Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 12/24/2021 00:00:00
 Ending Date 12/25/2021 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.052	65.310
Evaporation Loss	0.000	0.000
Infiltration Loss	0.021	25.950
Surface Runoff	0.031	39.279
Final Storage	0.000	0.119
Continuity Error (%)	-0.057	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.031	0.314
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.031	0.314
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 25 YR DESIGN STORM EVENT

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
101	65.31	0.00	0.00	40.74	24.39	0.10	10.28	0.373
201	65.31	0.00	0.00	11.16	54.17	0.22	44.69	0.829

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.35	0.80	298.58	0 05:26	0.80

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow 10^6 ltr	Total Inflow 10^6 ltr	Flow Balance Error Percent
OF1	OUTFALL	10.28	10.28	0 02:30	0.0976	0.0976	0.000
OF2	OUTFALL	0.00	4.97	0 05:26	0	0.217	0.000
SU1	STORAGE	44.69	44.69	0 02:25	0.217	0.217	0.000

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt	Exfil Pcnt	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.041	12	0	0	0.129	38	0 05:26	4.97

Outfall Loading Summary

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 25 YR DESIGN STORM EVENT

Outfall Node	Flow	Avg	Max	Total
	Freq	Flow	Flow	Volume
	Pcnt	LPS	LPS	10^6 ltr
OF1	65.71	1.71	10.28	0.098
OF2	63.08	3.98	4.97	0.217
System	64.39	5.69	14.88	0.314

Link Flow Summary

Link	Type	Maximum	Time of Max	Maximum	Max/	Max/
		Flow	Occurrence	Veloc	Full	Full
		LPS	days hr:min	m/sec	Flow	Depth
OR1	ORIFICE	4.97	0 05:26			1.00

Flow Classification Summary

Conduit	Adjusted Length	Fraction of Time in Flow Class								
		/Actual Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Crit	Inlet Ltd
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Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Mar 04 16:25:38 2022

Analysis ended on: Fri Mar 04 16:25:38 2022

Total elapsed time: < 1 sec

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 50 YR DESIGN STORM EVENT

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

Element Count

Number of rain gages	5
Number of subcatchments ...	2
Number of nodes	3
Number of links	1
Number of pollutants	0
Number of land uses	0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_6h_38.8mm_2yr	SCS_6h_38.8mm_2yr	INTENSITY	5 min.
SCS_6h_49.4mm_5yr	SCS_6h_49.4mm_5yr	INTENSITY	5 min.
SCS_6h_65.3mm_25yr	SCS_6h_65.3mm_25yr	INTENSITY	5 min.
SCS_6h_71.9mm_50yr	SCS_6h_71.9mm_50yr	INTENSITY	5 min.
SCS_6h_78.4mm_100yr	SCS_6h_78.4mm_100yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	0.40	85.00	0.00	1.0000	SCS_6h_71.9mm_50yr	OF1
201	0.40	85.00	68.40	1.0000	SCS_6h_71.9mm_50yr	SU1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	297.78	1.06	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
OR1	SU1	OF2	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
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NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 50 YR DESIGN STORM EVENT

 Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 12/24/2021 00:00:00
 Ending Date 12/25/2021 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.058	71.911
Evaporation Loss	0.000	0.000
Infiltration Loss	0.022	27.445
Surface Runoff	0.036	44.389
Final Storage	0.000	0.119
Continuity Error (%)	-0.058	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.036	0.355
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.036	0.355
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 50 YR DESIGN STORM EVENT

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
101	71.91	0.00	0.00	43.16	28.58	0.11	12.94	0.397
201	71.91	0.00	0.00	11.73	60.20	0.24	49.90	0.837

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.40	0.84	298.62	0 05:27	0.84

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow 10^6 ltr	Total Inflow 10^6 ltr	Flow Balance Volume	Error Percent
OF1	OUTFALL	12.94	12.94	0 02:30	0.114	0.114	0.000	0.000
OF2	OUTFALL	0.00	5.09	0 05:27	0	0.241	0.000	0.000
SU1	STORAGE	49.90	49.90	0 02:25	0.241	0.241	0.000	0.000

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt	Exfil Pcnt	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.051	15	0	0	0.151	44	0 05:27	5.09

Outfall Loading Summary

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 50 YR DESIGN STORM EVENT

Outfall Node	Flow	Avg	Max	Total
	Freq	Flow	Flow	Volume
	Pcnt	LPS	LPS	10^6 ltr
OF1	66.08	1.99	12.94	0.114
OF2	68.09	4.09	5.09	0.241
System	67.09	6.09	17.65	0.355

Link Flow Summary

Link	Type	Maximum	Time of Max	Maximum	Max/	Max/
		Flow	Occurrence	Veloc	Full	Full
		LPS	days hr:min	m/sec	Flow	Depth
OR1	ORIFICE	5.09	0 05:27			1.00

Flow Classification Summary

Conduit	Adjusted Length	Fraction of Time in Flow Class								
		/Actual Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Crit	Inlet Ltd
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Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Mar 04 16:26:13 2022

Analysis ended on: Fri Mar 04 16:26:13 2022

Total elapsed time: < 1 sec

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 100 YR DESIGN STORM EVENT

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

Element Count

Number of rain gages	5
Number of subcatchments ...	2
Number of nodes	3
Number of links	1
Number of pollutants	0
Number of land uses	0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_6h_38.8mm_2yr	SCS_6h_38.8mm_2yr	INTENSITY	5 min.
SCS_6h_49.4mm_5yr	SCS_6h_49.4mm_5yr	INTENSITY	5 min.
SCS_6h_65.3mm_25yr	SCS_6h_65.3mm_25yr	INTENSITY	5 min.
SCS_6h_71.9mm_50yr	SCS_6h_71.9mm_50yr	INTENSITY	5 min.
SCS_6h_78.4mm_100yr	SCS_6h_78.4mm_100yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	0.40	85.00	0.00	1.0000	SCS_6h_78.4mm_100yr	OF1
201	0.40	85.00	68.40	1.0000	SCS_6h_78.4mm_100yr	SU1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	297.78	1.06	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
OR1	SU1	OF2	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
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NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 100 YR DESIGN STORM EVENT

 Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 12/24/2021 00:00:00
 Ending Date 12/25/2021 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.063	78.413
Evaporation Loss	0.000	0.000
Infiltration Loss	0.023	28.820
Surface Runoff	0.040	49.521
Final Storage	0.000	0.119
Continuity Error (%)	-0.059	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.040	0.396
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.040	0.396
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 4.50 sec
 Average Time Step : 5.00 sec
 Maximum Time Step : 5.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 100 YR DESIGN STORM EVENT

Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
101	78.41	0.00	0.00	45.36	32.88	0.13	15.81	0.419
201	78.41	0.00	0.00	12.28	66.16	0.26	55.11	0.844

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.45	0.87	298.65	0 05:28	0.87

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow 10^6 ltr	Total Inflow 10^6 ltr	Flow Balance Error Percent
OF1	OUTFALL	15.81	15.81	0 02:30	0.132	0.132	0.000
OF2	OUTFALL	0.00	5.19	0 05:28	0	0.265	0.000
SU1	STORAGE	55.11	55.11	0 02:25	0.265	0.265	0.000

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt	Exfil Pcnt	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.062	18	0	0	0.172	50	0 05:28	5.19

Outfall Loading Summary

WILSON DEVELOPMENT – EAST RIDGE STORAGE SITE – 100 YR DESIGN STORM EVENT

Outfall Node	Flow	Avg	Max	Total
	Freq	Flow	Flow	Volume
	Pcnt	LPS	LPS	10^6 ltr
OF1	66.25	2.29	15.81	0.132
OF2	73.01	4.20	5.19	0.265
System	69.63	6.49	20.63	0.396

Link Flow Summary

Link	Type	Maximum	Time of Max	Maximum	Max/	Max/
		Flow	Occurrence	Veloc	Full	Full
		LPS	days hr:min	m/sec	Flow	Depth
OR1	ORIFICE	5.19	0 05:28			1.00

Flow Classification Summary

Conduit	Adjusted Length	Fraction of Time in Flow Class								
		/Actual Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Crit	Inlet Ltd
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Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Mar 04 16:26:52 2022

Analysis ended on: Fri Mar 04 16:26:53 2022

Total elapsed time: 00:00:01